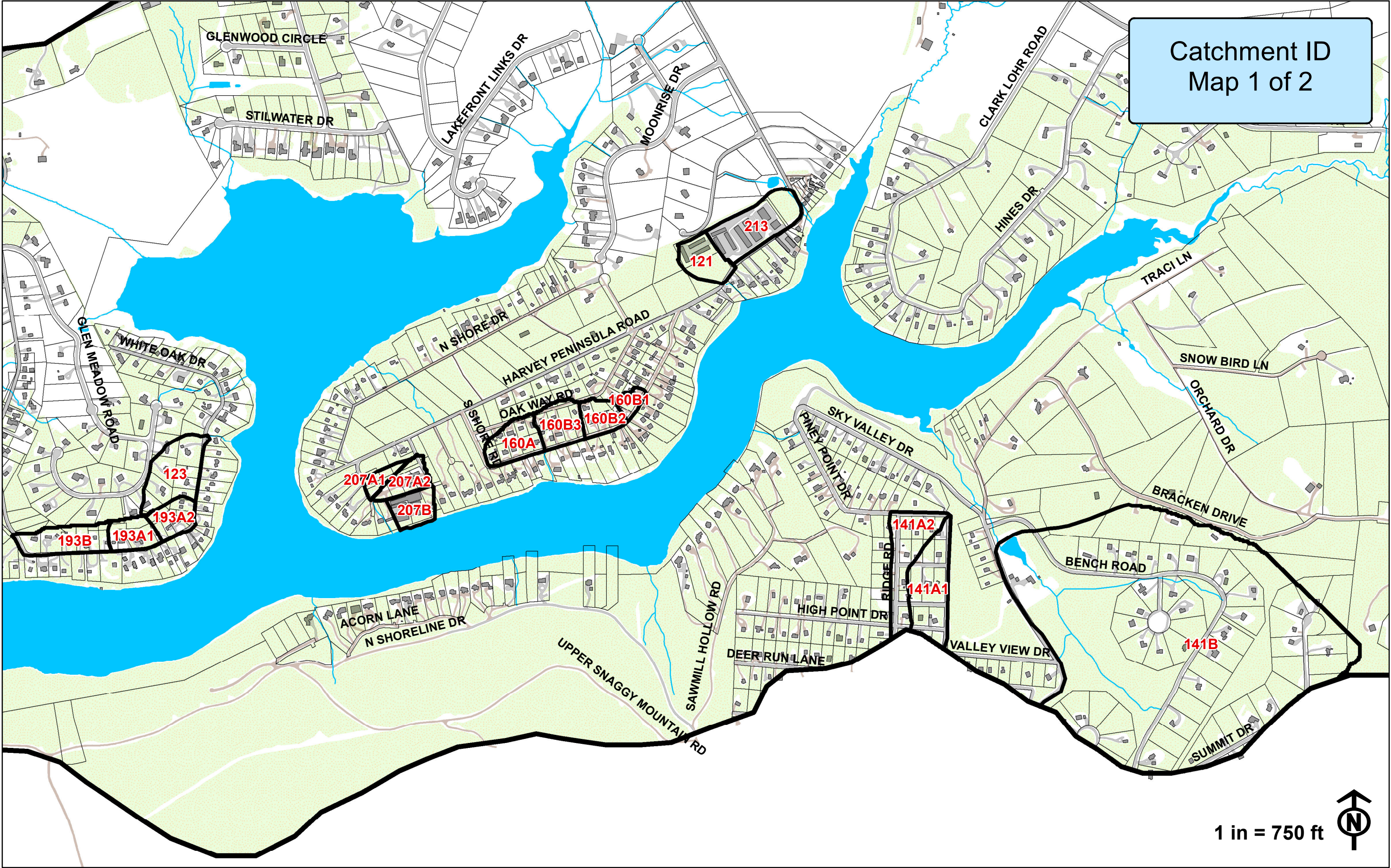
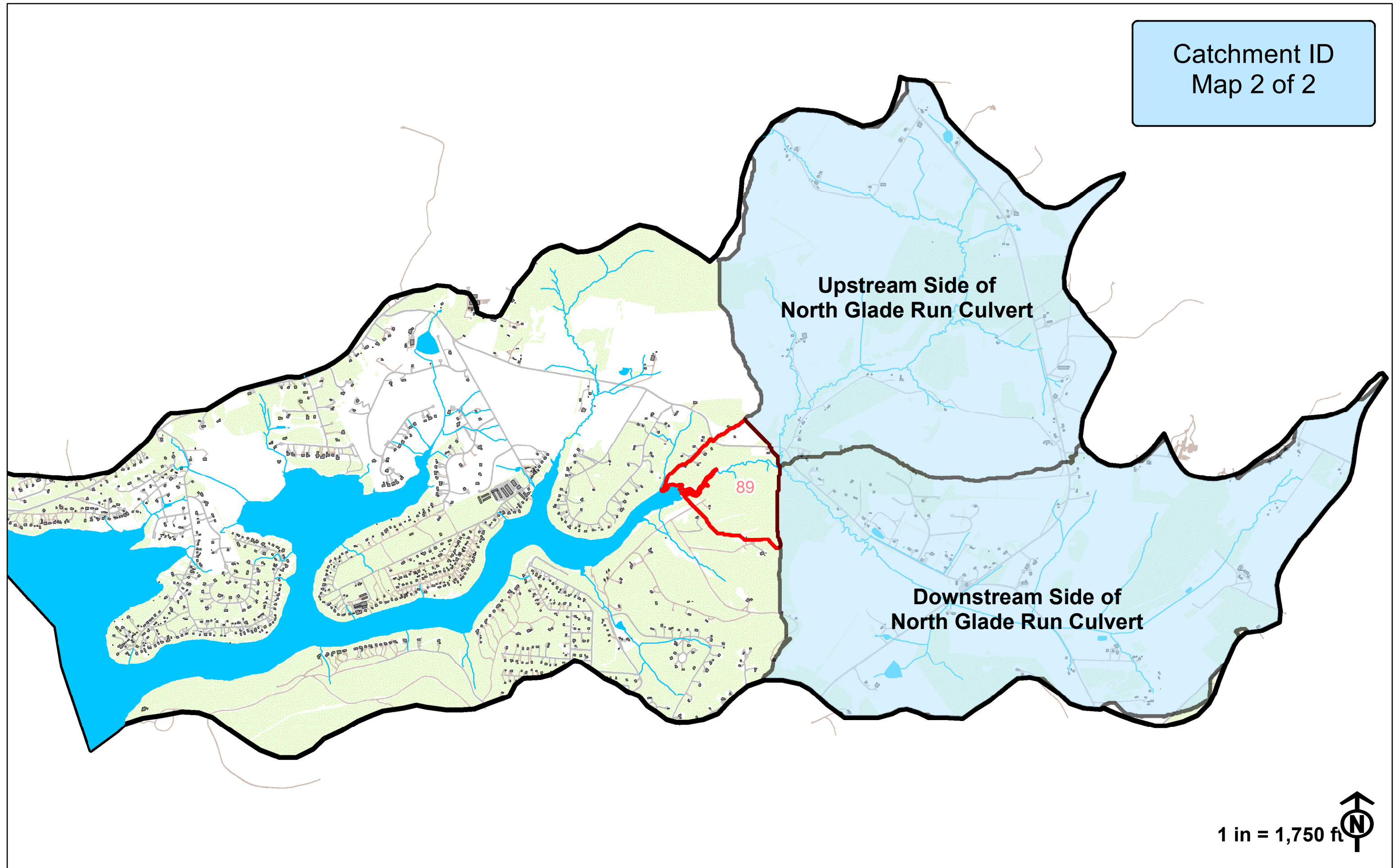


Catchment ID
Map 1 of 2







NOAA Atlas 14, Volume 2, Version 3
Location name: Swanton, Maryland, US*
Latitude: 39.4999°, Longitude: -79.2811°
Elevation: 2600 ft*
* source: Google Maps



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aeriels](#)

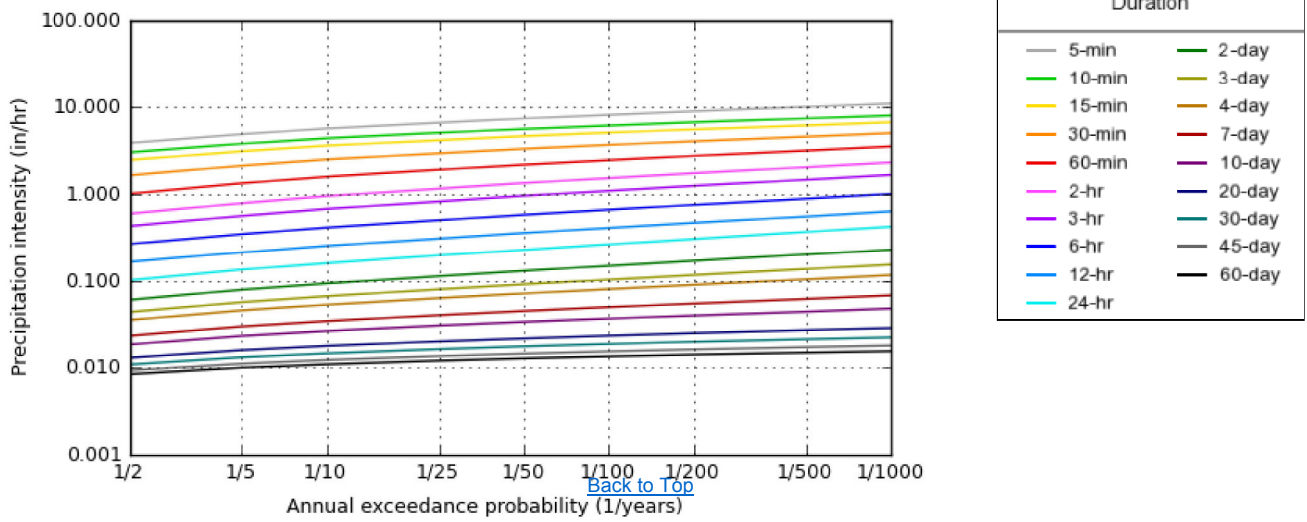
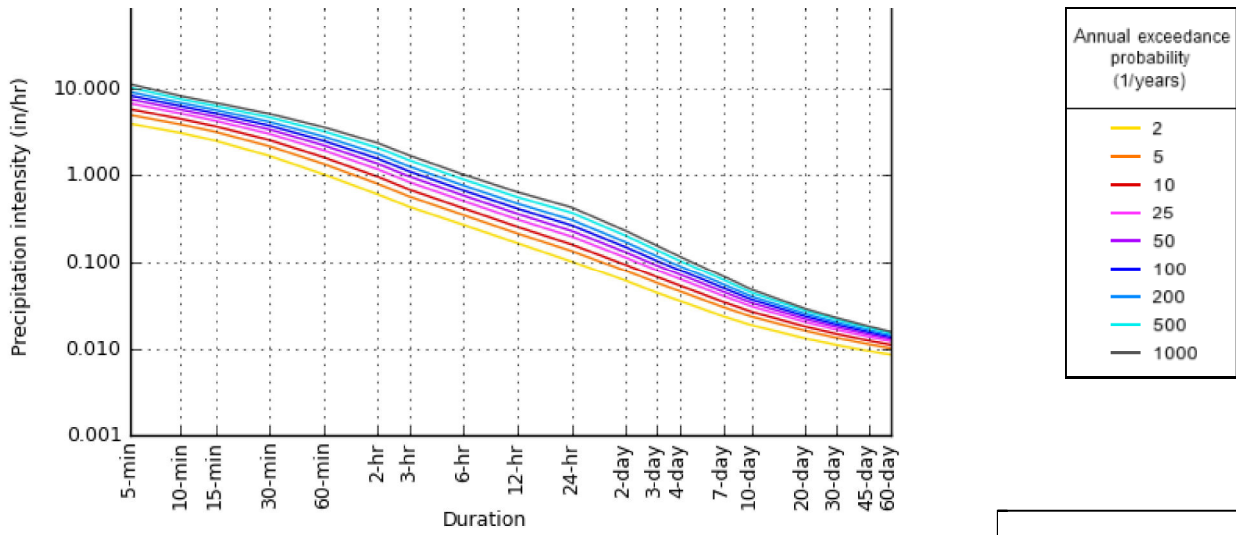
PF tabular

AMS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)¹									
Duration	Annual exceedance probability (1/years)								
	1/2	1/5	1/10	1/25	1/50	1/100	1/200	1/500	1/1000
5-min	3.90 (3.50–4.34)	4.91 (4.40–5.47)	5.72 (5.11–6.36)	6.66 (5.93–7.39)	7.45 (6.59–8.24)	8.21 (7.24–9.06)	9.02 (7.92–9.97)	10.1 (8.81–11.2)	11.1 (9.54–12.2)
10-min	3.04 (2.74–3.40)	3.82 (3.43–4.25)	4.42 (3.95–4.91)	5.10 (4.53–5.66)	5.65 (5.00–6.24)	6.18 (5.45–6.82)	6.74 (5.91–7.45)	7.46 (6.47–8.22)	8.09 (6.95–8.92)
15-min	2.48 (2.23–2.77)	3.13 (2.80–3.48)	3.62 (3.24–4.02)	4.20 (3.73–4.66)	4.66 (4.13–5.15)	5.12 (4.51–5.65)	5.59 (4.90–6.17)	6.20 (5.38–6.84)	6.74 (5.79–7.43)
30-min	1.66 (1.49–1.85)	2.14 (1.92–2.39)	2.52 (2.25–2.80)	2.97 (2.64–3.29)	3.33 (2.95–3.68)	3.70 (3.26–4.08)	4.08 (3.57–4.50)	4.60 (3.99–5.07)	5.05 (4.34–5.57)
60-min	1.02 (0.917–1.14)	1.34 (1.21–1.50)	1.60 (1.43–1.78)	1.92 (1.71–2.13)	2.20 (1.94–2.43)	2.47 (2.18–2.73)	2.77 (2.43–3.06)	3.18 (2.76–3.50)	3.55 (3.05–3.91)
2-hr	0.601 (0.538–0.669)	0.793 (0.710–0.882)	0.956 (0.853–1.06)	1.17 (1.04–1.29)	1.35 (1.19–1.49)	1.54 (1.35–1.70)	1.75 (1.53–1.93)	2.06 (1.77–2.26)	2.33 (1.98–2.56)
3-hr	0.432 (0.389–0.484)	0.567 (0.510–0.633)	0.681 (0.610–0.758)	0.830 (0.739–0.920)	0.963 (0.852–1.07)	1.10 (0.969–1.22)	1.26 (1.09–1.38)	1.48 (1.27–1.62)	1.68 (1.43–1.84)
6-hr	0.268 (0.240–0.303)	0.348 (0.310–0.393)	0.416 (0.370–0.468)	0.505 (0.446–0.566)	0.583 (0.513–0.653)	0.666 (0.582–0.745)	0.759 (0.656–0.846)	0.891 (0.761–0.992)	1.01 (0.856–1.13)
12-hr	0.165 (0.148–0.188)	0.213 (0.191–0.243)	0.255 (0.226–0.289)	0.310 (0.274–0.350)	0.359 (0.315–0.404)	0.412 (0.358–0.463)	0.471 (0.405–0.527)	0.557 (0.472–0.622)	0.638 (0.534–0.711)
24-hr	0.101 (0.093–0.112)	0.134 (0.123–0.148)	0.160 (0.145–0.176)	0.197 (0.178–0.217)	0.229 (0.204–0.251)	0.265 (0.234–0.291)	0.306 (0.266–0.337)	0.370 (0.315–0.408)	0.426 (0.356–0.471)
2-day	0.060 (0.055–0.066)	0.079 (0.072–0.086)	0.093 (0.085–0.101)	0.113 (0.102–0.123)	0.130 (0.117–0.142)	0.149 (0.132–0.163)	0.170 (0.149–0.186)	0.202 (0.173–0.222)	0.229 (0.194–0.254)
3-day	0.043 (0.040–0.047)	0.056 (0.052–0.061)	0.066 (0.061–0.072)	0.080 (0.073–0.086)	0.091 (0.082–0.098)	0.103 (0.092–0.112)	0.116 (0.103–0.127)	0.136 (0.119–0.150)	0.154 (0.132–0.170)
4-day	0.035 (0.033–0.038)	0.045 (0.042–0.049)	0.053 (0.049–0.057)	0.063 (0.058–0.068)	0.071 (0.065–0.077)	0.080 (0.073–0.086)	0.089 (0.080–0.097)	0.104 (0.091–0.113)	0.116 (0.101–0.128)
7-day	0.023 (0.022–0.025)	0.030 (0.028–0.032)	0.034 (0.032–0.037)	0.040 (0.037–0.043)	0.045 (0.041–0.048)	0.049 (0.045–0.053)	0.054 (0.050–0.059)	0.061 (0.055–0.067)	0.068 (0.060–0.074)
10-day	0.018 (0.017–0.020)	0.023 (0.022–0.025)	0.026 (0.025–0.028)	0.030 (0.029–0.032)	0.033 (0.031–0.035)	0.036 (0.034–0.039)	0.040 (0.037–0.042)	0.044 (0.040–0.047)	0.048 (0.043–0.052)
20-day	0.013 (0.012–0.014)	0.016 (0.015–0.017)	0.018 (0.017–0.019)	0.020 (0.019–0.021)	0.022 (0.021–0.023)	0.023 (0.022–0.025)	0.025 (0.023–0.026)	0.027 (0.025–0.029)	0.028 (0.026–0.031)
30-day	0.011 (0.010–0.011)	0.013 (0.013–0.014)	0.015 (0.014–0.015)	0.016 (0.016–0.017)	0.018 (0.017–0.018)	0.019 (0.018–0.020)	0.020 (0.019–0.021)	0.021 (0.020–0.023)	0.022 (0.021–0.024)
45-day	0.009 (0.009–0.010)	0.011 (0.011–0.012)	0.012 (0.012–0.013)	0.014 (0.013–0.014)	0.015 (0.014–0.015)	0.015 (0.015–0.016)	0.016 (0.015–0.017)	0.017 (0.016–0.018)	0.018 (0.017–0.019)
60-day	0.008 (0.008–0.009)	0.010 (0.010–0.010)	0.011 (0.010–0.011)	0.012 (0.012–0.013)	0.013 (0.012–0.013)	0.013 (0.013–0.014)	0.014 (0.013–0.015)	0.015 (0.014–0.016)	0.015 (0.015–0.016)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of annual maxima series (AMS).
 Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and annual exceedance probability) will be greater than the upper bound (or less than the lower bound) is 5%.
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PF graphical

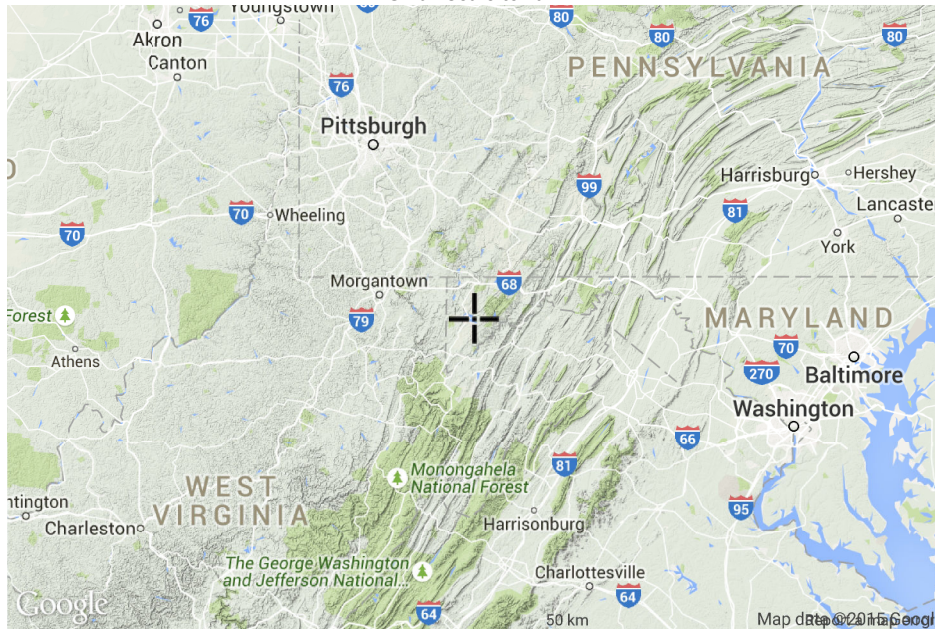


Maps & aerials

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Small scale terrain



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NOAA Atlas 14, Volume 2, Version 3
Location name: Swanton, Maryland, US*
Latitude: 39.4999°, Longitude: -79.2811°
Elevation: 2600 ft*
* source: Google Maps



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M. Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

PF tabular

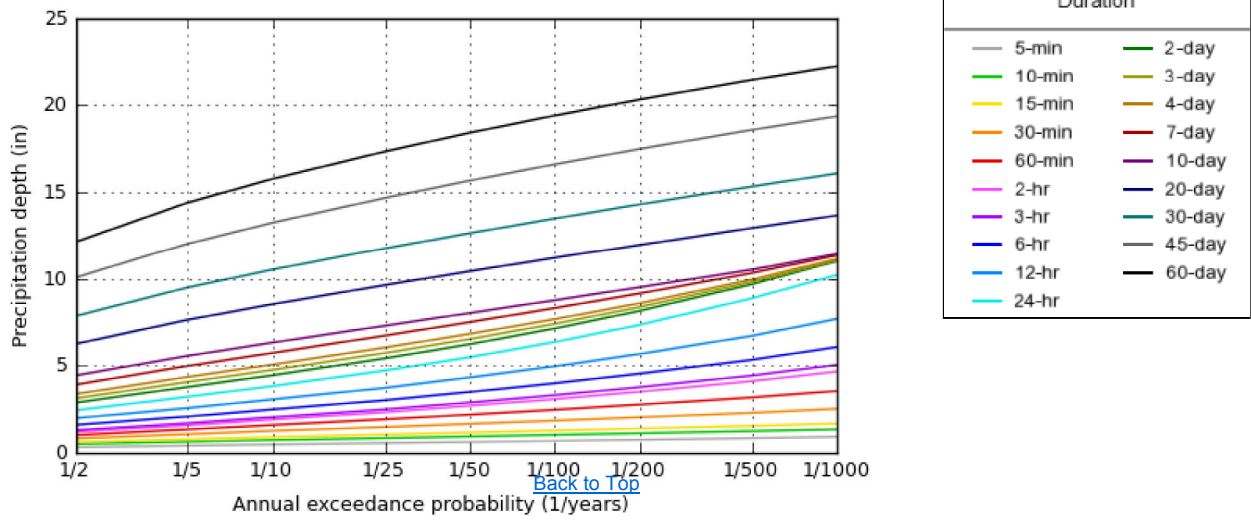
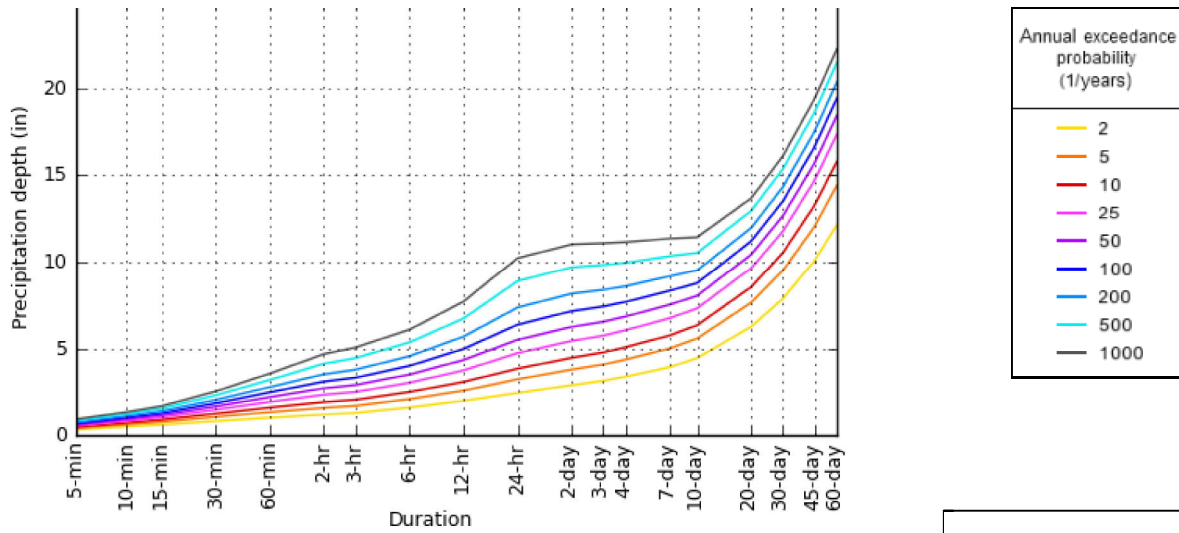
AMS-based point precipitation frequency estimates with 90% confidence intervals (in inches)

Duration	Annual exceedance probability (1/years)								
	1/2	1/5	1/10	1/25	1/50	1/100	1/200	1/500	1/1000
5-min	0.325 (0.292-0.362)	0.409 (0.367-0.456)	0.477 (0.426-0.530)	0.555 (0.494-0.616)	0.621 (0.549-0.687)	0.684 (0.603-0.755)	0.752 (0.660-0.831)	0.845 (0.734-0.932)	0.926 (0.795-1.02)
10-min	0.507 (0.456-0.567)	0.636 (0.571-0.709)	0.736 (0.658-0.818)	0.850 (0.755-0.943)	0.942 (0.833-1.04)	1.03 (0.908-1.14)	1.12 (0.985-1.24)	1.24 (1.08-1.37)	1.35 (1.16-1.49)
15-min	0.620 (0.558-0.693)	0.782 (0.701-0.871)	0.906 (0.809-1.01)	1.05 (0.933-1.17)	1.17 (1.03-1.29)	1.28 (1.13-1.41)	1.40 (1.23-1.54)	1.55 (1.35-1.71)	1.69 (1.45-1.86)
30-min	0.830 (0.747-0.927)	1.07 (0.960-1.19)	1.26 (1.13-1.40)	1.48 (1.32-1.64)	1.67 (1.48-1.84)	1.85 (1.63-2.04)	2.04 (1.79-2.25)	2.30 (1.99-2.53)	2.53 (2.17-2.79)
60-min	1.02 (0.917-1.14)	1.34 (1.21-1.50)	1.60 (1.43-1.78)	1.92 (1.71-2.13)	2.20 (1.94-2.43)	2.47 (2.18-2.73)	2.77 (2.43-3.06)	3.18 (2.76-3.50)	3.55 (3.05-3.91)
2-hr	1.20 (1.08-1.34)	1.59 (1.42-1.76)	1.91 (1.71-2.12)	2.33 (2.07-2.58)	2.70 (2.38-2.98)	3.09 (2.70-3.40)	3.51 (3.05-3.86)	4.11 (3.54-4.52)	4.67 (3.97-5.12)
3-hr	1.30 (1.17-1.45)	1.70 (1.53-1.90)	2.05 (1.83-2.28)	2.49 (2.22-2.76)	2.89 (2.56-3.20)	3.31 (2.91-3.65)	3.77 (3.29-4.15)	4.44 (3.81-4.87)	5.05 (4.29-5.54)
6-hr	1.61 (1.44-1.82)	2.08 (1.86-2.35)	2.49 (2.21-2.80)	3.02 (2.67-3.39)	3.49 (3.07-3.91)	3.99 (3.48-4.46)	4.55 (3.93-5.07)	5.34 (4.56-5.94)	6.07 (5.12-6.75)
12-hr	1.99 (1.78-2.27)	2.57 (2.30-2.92)	3.07 (2.73-3.48)	3.73 (3.30-4.21)	4.33 (3.80-4.87)	4.96 (4.31-5.57)	5.67 (4.88-6.35)	6.72 (5.69-7.50)	7.69 (6.43-8.56)
24-hr	2.44 (2.23-2.69)	3.22 (2.95-3.56)	3.83 (3.49-4.22)	4.72 (4.26-5.20)	5.49 (4.91-6.04)	6.36 (5.61-6.99)	7.35 (6.40-8.09)	8.87 (7.55-9.79)	10.2 (8.54-11.3)
2-day	2.88 (2.66-3.15)	3.78 (3.48-4.13)	4.45 (4.08-4.86)	5.42 (4.92-5.92)	6.23 (5.61-6.81)	7.14 (6.35-7.81)	8.15 (7.15-8.93)	9.68 (8.32-10.7)	11.0 (9.30-12.2)
3-day	3.13 (2.90-3.39)	4.07 (3.77-4.40)	4.76 (4.40-5.15)	5.73 (5.25-6.20)	6.53 (5.93-7.08)	7.41 (6.66-8.05)	8.37 (7.43-9.12)	9.82 (8.55-10.8)	11.1 (9.48-12.2)
4-day	3.37 (3.15-3.63)	4.36 (4.06-4.68)	5.07 (4.71-5.44)	6.04 (5.58-6.49)	6.83 (6.25-7.36)	7.67 (6.96-8.29)	8.59 (7.71-9.31)	9.95 (8.77-10.9)	11.1 (9.66-12.3)
7-day	3.91 (3.67-4.19)	4.98 (4.67-5.33)	5.73 (5.36-6.14)	6.73 (6.26-7.21)	7.50 (6.94-8.05)	8.31 (7.63-8.94)	9.15 (8.33-9.88)	10.3 (9.28-11.2)	11.4 (10.1-12.4)
10-day	4.44 (4.20-4.71)	5.56 (5.26-5.90)	6.33 (5.97-6.70)	7.30 (6.85-7.73)	8.03 (7.50-8.52)	8.76 (8.15-9.32)	9.51 (8.79-10.2)	10.5 (9.62-11.3)	11.4 (10.4-12.6)
20-day	6.25 (5.96-6.58)	7.64 (7.28-8.04)	8.53 (8.12-8.98)	9.63 (9.15-10.2)	10.4 (9.87-11.0)	11.2 (10.6-11.9)	12.0 (11.2-12.7)	12.9 (12.1-13.8)	13.7 (12.6-14.7)
30-day	7.85 (7.51-8.23)	9.50 (9.08-9.95)	10.5 (10.1-11.0)	11.8 (11.2-12.3)	12.7 (12.0-13.3)	13.5 (12.8-14.2)	14.3 (13.5-15.1)	15.3 (14.4-16.3)	16.1 (15.0-17.1)
45-day	10.1 (9.67-10.5)	12.0 (11.6-12.6)	13.3 (12.7-13.9)	14.7 (14.0-15.4)	15.7 (15.0-16.4)	16.6 (15.8-17.4)	17.5 (16.6-18.4)	18.6 (17.5-19.6)	19.4 (18.2-20.5)
60-day	12.1 (11.6-12.7)	14.4 (13.8-15.0)	15.8 (15.1-16.5)	17.3 (16.6-18.1)	18.4 (17.6-19.3)	19.4 (18.5-20.3)	20.3 (19.3-21.3)	21.5 (20.3-22.6)	22.2 (21.0-23.5)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of annual maxima series (AMS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and annual exceedance probability) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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Large scale terrain



Large scale aerial



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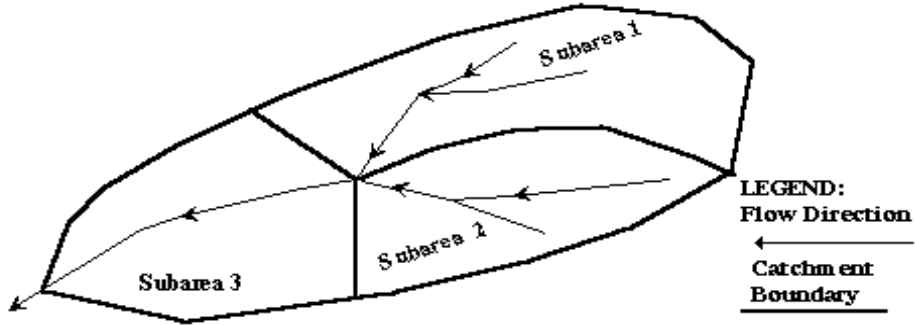
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Area-Weighting for Runoff Coefficient Calculation

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 121

Illustration



Instructions: For each catchment subarea, enter values for A and C.

Subarea ID	Area acres	Runoff Coeff.	Product
	A	C*	CA
input	input	input	output
121a	1.34	0.89	1.19
121b	1.73	0.17	0.29
Sum:	3.07	Sum:	1.49

Area-Weighted Runoff Coefficient (sum CA/sum A) = 0.48

*See sheet "Design Info" for imperviousness-based runoff coefficient values.

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
Catchment ID: Subcatchment ID 121

I. Catchment Hydrologic Data

Catchment ID = 121
 Area = 3.07 Acres
 Percent Imperviousness = 21.50 %
 NRCS Soil Type = B A, B, C, or D

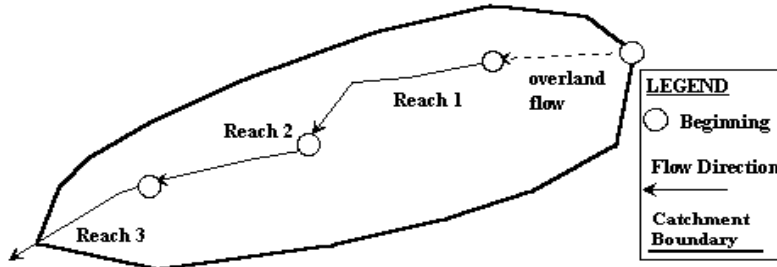
II. Rainfall Information $I \text{ (inch/hr)} = C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.48 (enter an override C value if desired, or leave blank to accept calculated C.)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time Tf minutes output
	ft/ft input	ft input				
Overland	0.0220	85	0.48	N/A	0.18	7.96
1	0.3534	523		10.00	5.94	1.47
2	0.0010	342		20.00	0.63	9.00
3						
4						
5						
Sum		950				
Computed Tc =						18.43

IV. Peak Runoff Prediction

Rainfall Intensity at Computed Tc, I = 3.33 inch/hr

Peak Flowrate, Q_p = 4.90 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
Catchment ID: Subcatchment ID 123

I. Catchment Hydrologic Data

Catchment ID = 123
 Area = 5.58 Acres
 Percent Imperviousness = 12.10 %
 NRCS Soil Type = B A, B, C, or D

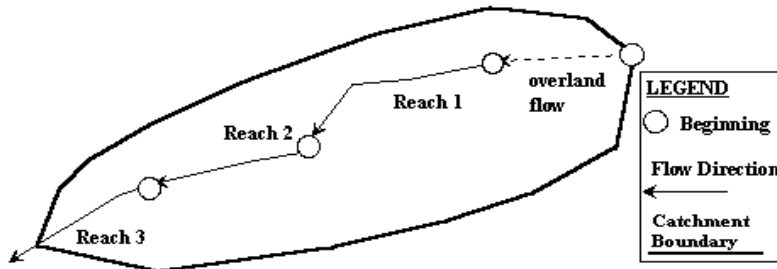
II. Rainfall Information $I \text{ (inch/hr)} = C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.26 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff C-5	NRCS Conveyance	Flow Velocity V	Flow Time Tf	
	ft/ft input	ft input	output	input	fps output	minutes output	
Overland	0.0455	87	0.26	N/A	0.17	8.60	
1	0.0854	804		10.00	2.92	4.58	
2							
3							
4							
5							
Sum		891					
						Computed T_c =	13.18

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c , I = 3.90 inch/hr

Peak Flowrate, Q_p = 5.66 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 141A1

I. Catchment Hydrologic Data

Catchment ID = 141A1
 Area = 6.58 Acres
 Percent Imperviousness = 22.30 %
 NRCS Soil Type = B A, B, C, or D

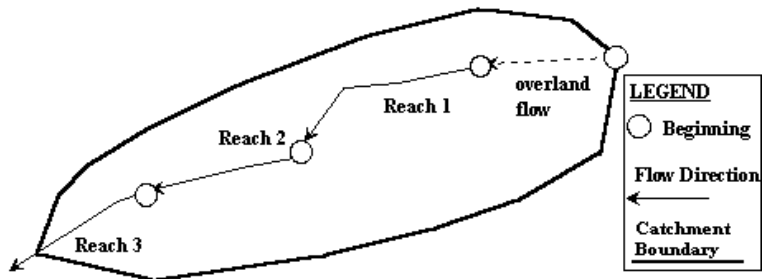
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.28 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time Tf minutes output
Overland	0.0693	129	0.28	N/A	0.24	8.86
1	0.1141	1,118		20.00	6.75	2.76
2						
3						
4						
5						
Sum		1,246				
					Computed Tc =	11.62

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c , I = 4.12 inch/hr

Peak Flowrate, Q_p = 7.59 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 141A2

I. Catchment Hydrologic Data

Catchment ID = 141A2
 Area = 5.78 Acres
 Percent Imperviousness = 23.00 %
 NRCS Soil Type = B A, B, C, or D

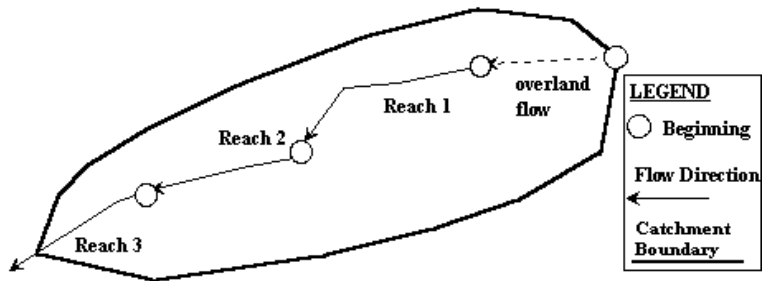
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.28 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff $C-5$	NRCS Conveyance	Flow Velocity V	Flow Time T_f
	ft/ft input	ft input	output		fps output	minutes output
Overland	0.0784	138	0.28	N/A	0.26	8.82
1	0.0987	1,293		20.00	6.28	3.43
2						
3						
4						
5						
Sum		1,431			Computed T_c =	12.25

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c , I = 4.03 inch/hr

Peak Flowrate, Q_p = 6.52 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 141B

I. Catchment Hydrologic Data

Catchment ID = 141B
 Area = 111.96 Acres
 Percent Imperviousness = 6.50 %
 NRCS Soil Type = B A, B, C, or D

For catchments larger than 90 acres, CUHP hydrograph and routing are recommended.

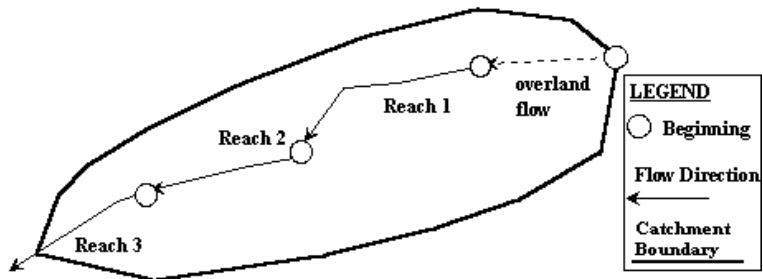
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.18 (enter an override C value if desired, or leave blank to accept calculated C.)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
	ft/ft input	ft input				
Overland	0.0809	207	0.18	N/A	0.29	12.00
1	0.1177	3,140		10.00	3.43	15.25
2						
3						
4						
5						
Sum		3,347				
Computed T _c =						27.25

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 2.69 inch/hr

Peak Flowrate, Q_p = 54.28 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 160A

I. Catchment Hydrologic Data

Catchment ID = 160A
 Area = 3.35 Acres
 Percent Imperviousness = 22.80 %
 NRCS Soil Type = B A, B, C, or D

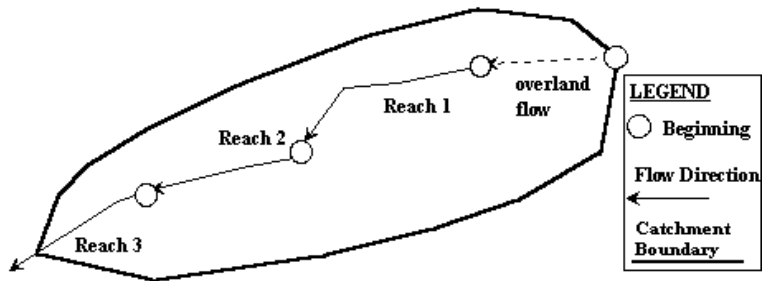
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.30 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff $C-5$	NRCS Conveyance	Flow Velocity V	Flow Time T_f
	ft/ft input	ft input	output	input	fps output	minutes output
Overland	0.0310	75	0.30	N/A	0.14	8.58
1	0.0937	293		7.00	2.14	2.28
2	0.0260	264		15.00	2.42	1.82
3						
4						
5						
Sum		632				
Computed T_c =						12.68

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c , I = 3.97 inch/hr

Peak Flowrate, Q_p = 3.99 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 160B

I. Catchment Hydrologic Data

Catchment ID = 160B
 Area = 6.48 Acres
 Percent Imperviousness = 24.10 %
 NRCS Soil Type = A A, B, C, or D

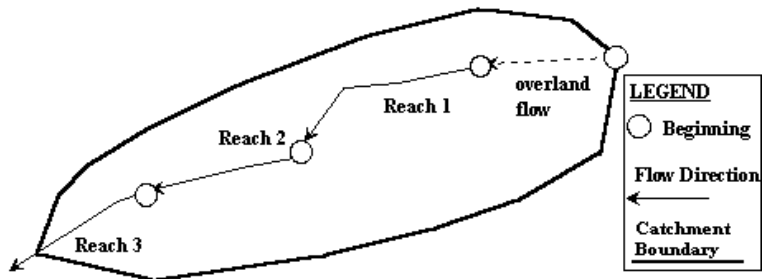
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.26 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0598	96	0.26	N/A	0.19	8.24
1	0.0508	303		15.00	3.38	1.49
2	0.0276	357		15.00	2.49	2.39
3	0.0175	352		15.00	1.99	2.95
4						
5						
Sum		1,108				
					Computed T _c =	15.07

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 3.67 inch/hr

Peak Flowrate, Q_p = 6.19 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 160B1

I. Catchment Hydrologic Data

Catchment ID = 160B1
 Area = 0.96 Acres
 Percent Imperviousness = 30.20 %
 NRCS Soil Type = A A, B, C, or D

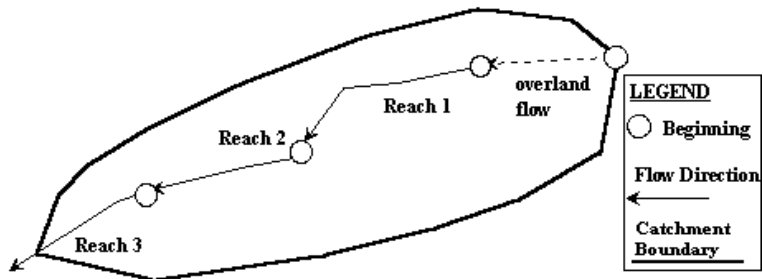
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.26 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0598	96	0.26	N/A	0.19	8.24
1	0.0508	303		15.00	3.38	1.49
2						
3						
4						
5						
Sum		399				
Computed T _c =						9.73

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 4.43 inch/hr

Peak Flowrate, Q_p = 1.11 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 160B2

I. Catchment Hydrologic Data

Catchment ID = 160B2
 Area = 2.30 Acres
 Percent Imperviousness = 21.60 %
 NRCS Soil Type = A A, B, C, or D

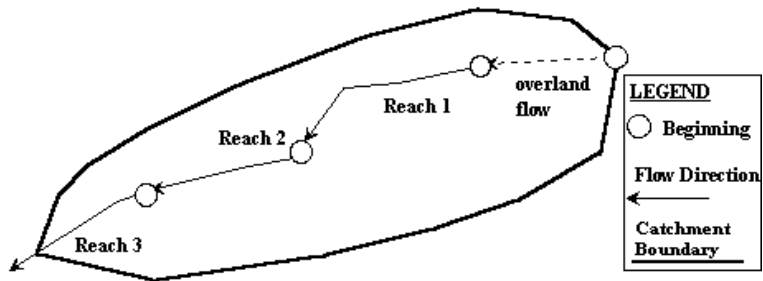
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.26 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0394	56	0.26	N/A	0.13	7.20
1	0.1273	154		7.00	2.50	1.03
2	0.0276	357		15.00	2.49	2.39
3						
4						
5						
Sum		567				
					Computed T _c =	10.62

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 4.28 inch/hr

Peak Flowrate, Q_p = 2.56 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 160B3

I. Catchment Hydrologic Data

Catchment ID = 160B3
 Area = 3.23 Acres
 Percent Imperviousness = 24.10 %
 NRCS Soil Type = A A, B, C, or D

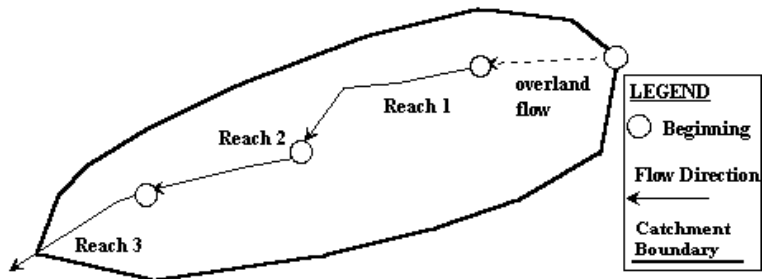
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.26 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0102	72	0.26	N/A	0.09	12.79
1	0.1000	289		7.00	2.21	2.18
2	0.0175	352		15.00	1.99	2.95
3						
4						
5						
Sum		713				
Computed T _c =						17.92

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 3.37 inch/hr

Peak Flowrate, Q_p = 2.83 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 193A

I. Catchment Hydrologic Data

Catchment ID = 193A
 Area = 4.95 Acres
 Percent Imperviousness = 14.10 %
 NRCS Soil Type = B A, B, C, or D

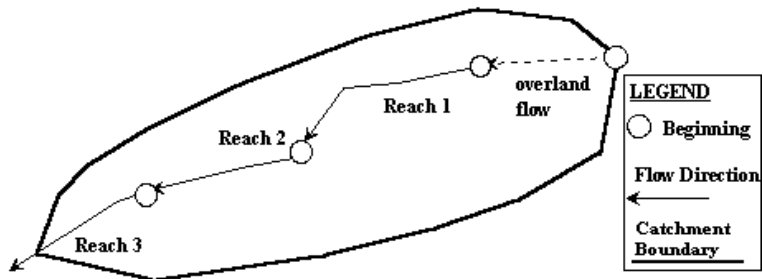
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.28 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0384	61	0.28	N/A	0.14	7.42
1	0.1020	219		7.00	2.24	1.63
2	0.0188	435		20.00	2.74	2.64
3	0.0235	290		20.00	3.07	1.57
4						
5						
Sum		1,003		Computed T _c =		13.27

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 3.89 inch/hr

Peak Flowrate, Q_p = 5.39 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 193A1

I. Catchment Hydrologic Data

Catchment ID = 193A1
 Area = 2.50 Acres
 Percent Imperviousness = 13.50 %
 NRCS Soil Type = B A, B, C, or D

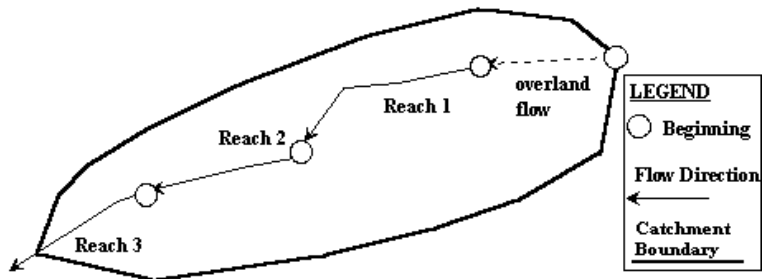
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.28 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time Tf minutes output
Overland	0.0384	61	0.28	N/A	0.14	7.42
1	0.1020	219		7.00	2.24	1.63
2	0.0188	435		20.00	2.74	2.64
3						
4						
5						
Sum		714				
					Computed Tc =	11.69

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c , I = 4.11 inch/hr

Peak Flowrate, Q_p = 2.88 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 193A2

I. Catchment Hydrologic Data

Catchment ID = 193A2
 Area = 2.45 Acres
 Percent Imperviousness = 14.70 %
 NRCS Soil Type = B A, B, C, or D

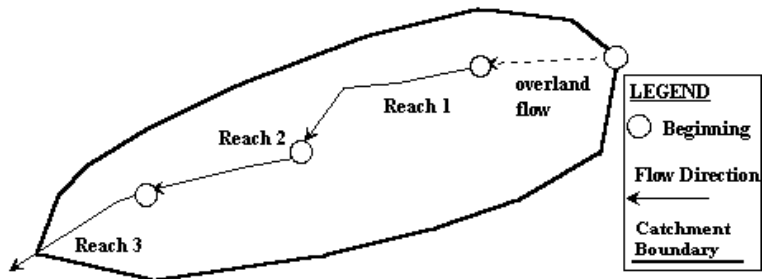
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.28 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0749	82	0.28	N/A	0.20	6.90
1	0.1470	168		20.00	7.67	0.36
2						
3						
4						
5						
Sum		250				
Computed T _c =						7.27

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 4.91 inch/hr

Peak Flowrate, Q_p = 3.37 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 193B

I. Catchment Hydrologic Data

Catchment ID = 193B
 Area = 4.13 Acres
 Percent Imperviousness = 15.90 %
 NRCS Soil Type = B A, B, C, or D

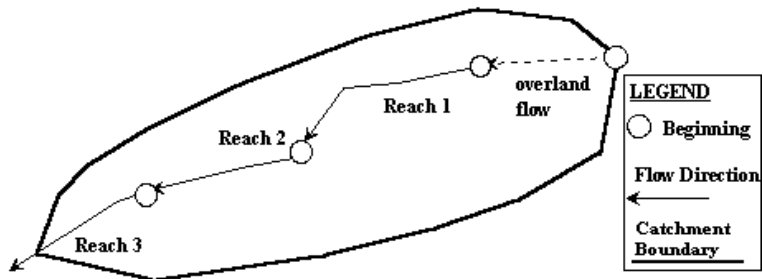
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.26 (enter an override C value if desired, or leave blank to accept calculated C.)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time Tf minutes output
	ft/ft input	ft input				
Overland	0.0452	59	0.26	N/A	0.14	7.05
1	0.1020	203		7.00	2.24	1.51
2	0.0173	510		15.00	1.98	4.30
3						
4						
5						
Sum		771				
Computed Tc =						12.87

IV. Peak Runoff Prediction

Rainfall Intensity at Computed Tc, I = 3.94 inch/hr

Peak Flowrate, Q_p = 4.24 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 207A1

I. Catchment Hydrologic Data

Catchment ID = 207A1
 Area = 1.11 Acres
 Percent Imperviousness = 8.40 %
 NRCS Soil Type = B A, B, C, or D

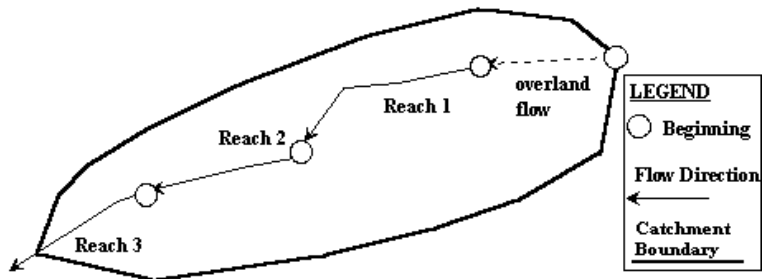
II. Rainfall Information $I \text{ (inch/hr)} = C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, $T_r =$ 10 years (input return period for design storm)
 $P1 =$ 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, $C =$ 0.26 (enter an override C value if desired, or leave blank to accept calculated C.)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0382	81	0.26	N/A	0.15	8.77
1	0.0490	334		7.00	1.55	3.59
2	0.0523	152		15.00	3.43	0.74
3						
4						
5						
Sum		567				
					Computed T _c =	13.11

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, $I =$ 3.91 inch/hr

Peak Flowrate, $Q_p =$ 1.13 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 207A2

I. Catchment Hydrologic Data

Catchment ID = 207A2
 Area = 2.21 Acres
 Percent Imperviousness = 31.20 %
 NRCS Soil Type = B A, B, C, or D

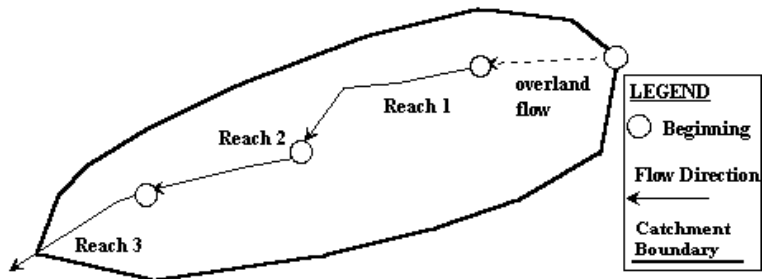
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.72 (enter an override C value if desired, or leave blank to accept calculated C.)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S ft/ft input	Length L ft input	5-yr Runoff Coeff C-5 output	NRCS Conveyance input	Flow Velocity V fps output	Flow Time T _f minutes output
Overland	0.0395	63	0.72	N/A	0.30	3.47
1	0.0340	152		7.00	1.29	1.97
2	0.0645	340		15.00	3.81	1.49
3						
4						
5						
Sum		555				
Computed T _c =						6.93

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c, I = 4.99 inch/hr

Peak Flowrate, Q_p = 7.94 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
 Catchment ID: Subcatchment ID 207B

I. Catchment Hydrologic Data

Catchment ID = 207B
 Area = 2.46 Acres
 Percent Imperviousness = 51.10 %
 NRCS Soil Type = B A, B, C, or D

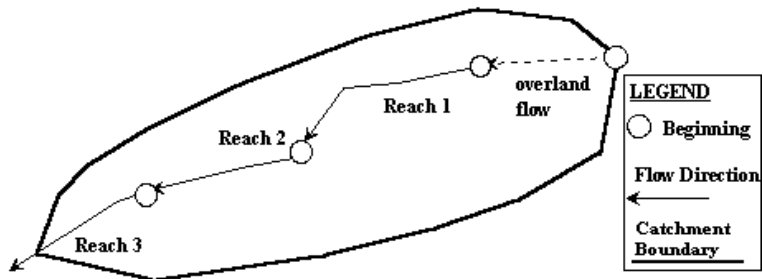
II. Rainfall Information I (inch/hr) = $C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.72 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff $C-5$	NRCS Conveyance	Flow Velocity V	Flow Time T_f
	ft/ft input	ft input	output	input	fps output	minutes output
Overland	0.0753	78	0.72	N/A	0.42	3.11
1	0.1417	379		7.00	2.64	2.40
2						
3						
4						
5						
Sum		457			Computed T_c =	5.51

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c , I = 5.34 inch/hr

Peak Flowrate, Q_p = 9.47 cfs

CALCULATION OF A PEAK RUNOFF USING RATIONAL METHOD

Project Title: North Glade Run Watershed
Catchment ID: Subcatchment ID 213

I. Catchment Hydrologic Data

Catchment ID = 213
 Area = 6.04 Acres
 Percent Imperviousness = 58.80 %
 NRCS Soil Type = B A, B, C, or D

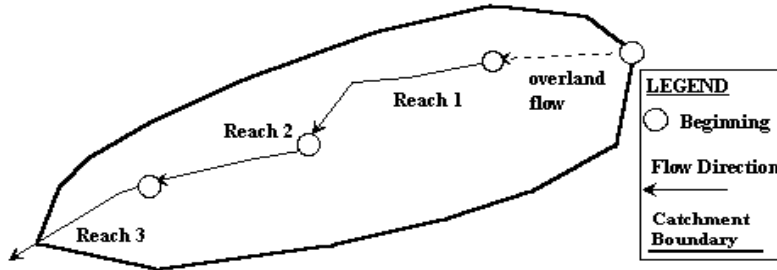
II. Rainfall Information $I \text{ (inch/hr)} = C1 * P1 / (C2 + Td)^{C3}$

Design Storm Return Period, T_r = 10 years (input return period for design storm)
 $P1$ = 1.60 inches (input one-hr precipitation--see Sheet "Design Info")

III. Analysis of Flow Time (Time of Concentration) for a Catchment

Override Runoff Coefficient, C = 0.72 (enter an override C value if desired, or leave blank to accept calculated C .)

Illustration



NRCS Land Type	Heavy Meadow	Tillage/Field	Short Pasture/Lawns	Nearly Bare Ground	Grassed Swales/Waterways	Paved Areas & Shallow Paved Swales (Sheet Flow)
Conveyance	2.5	5	7	10	15	20

Calculations:

Reach ID	Slope S	Length L	5-yr Runoff Coeff C-5	NRCS Conveyance	Flow Velocity V	Flow Time Tf
	ft/ft input	ft input	output	input	fps output	minutes output
Overland	0.0160	98	0.72	N/A	0.28	5.81
1	0.0415	619		20.00	4.08	2.53
2	0.0487	221		20.00	4.41	0.83
3						
4						
5						
Sum		938				
Computed Tc =						9.17

IV. Peak Runoff Prediction

Rainfall Intensity at Computed T_c , I = 4.53 inch/hr

Peak Flowrate, Q_p = 19.69 cfs

StreamStats Version 3 Beta

Flow Statistics Ungaged Site Report

Date: Tues Nov 17, 2015 10:58:02 AM GMT-5

Site Location: Maryland

NAD 1983 Latitude: 39.508 (39 30 29)

NAD 1983 Longitude: -79.2508 (-79 15 03)

Drainage Area: 1.64 mi²

Peak Flows Region Grid Basin Characteristics			
100% Peak Appalachian Plateau 2010 AHMMD (1.64 mi ²)			
Parameter	Value	Regression Equation Valid Range	
		Min	Max
Drainage Area (square miles)	1.64	0.52	293.7
Mean Basin Slope from 10m DEM ft per ft (feet per foot)	0.0882	0.06632	0.22653

Urban Flows Region Grid Basin Characteristics	
100% Undefined Region (1.64 mi ²)	

The selected watershed is entirely in an area for which flow equations were not defined.

Low Flows Region Grid Basin Characteristics	
100% Undefined Region (1.64 mi ²)	

The selected watershed is entirely in an area for which flow equations were not defined.

Peak Flows Region Grid Statistics						
Statistic	Value	Unit	Prediction Error (percent)	Equivalent years of record	90-Percent Prediction Interval	
					Min	Max
PK1 25	48	ft ³ /s	24	5.7		
PK1 5	60.7	ft ³ /s	22	5.9		
PK2	73.9	ft ³ /s	21	7.1		
PK5	126	ft ³ /s	22	12		
PK10	168	ft ³ /s	24	14		
PK25	233	ft ³ /s	29	15		
PK50	290	ft ³ /s	33	16		
PK100	356	ft ³ /s	37	15		
PK200	433	ft ³ /s	42	15		
PK500	552	ft ³ /s	48	15		

http://www.gishydro.umd.edu/HydroPanel/hydrology_panel_report_3rd_edition_final.pdf
http://www.gishydro.umd.edu/HydroPanel/hydrology_panel_report_3rd_edition_final.pdf

Thomas, Jr., W.O. and Moglen, G.E., 2010. An Update of Regional Regression Equations for Maryland. Appendix 3 in Application of Hydrologic Methods in Maryland. Third Edition. September 2010: Maryland State Highway Administration and Maryland Department of the Environment. 38 p.

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U.S. Department of the Interior | U.S. Geological Survey
 URL: http://streamstatsags.cr.usgs.gov/v3_beta/FTreport.htm

StreamStats Version 3 Beta

Flow Statistics Ungaged Site Report

Date: Thurs Nov 19, 2015 3:26:43 PM GMT-5

Site Location: Maryland

NAD 1983 Latitude: 39.507 (39 30 25)

NAD 1983 Longitude: -79.2532 (-79 15 12)

Drainage Area: 3.54 mi²

Peak Flows Region Grid Basin Characteristics

100% Peak Appalachian Plateau 2010 AHMMD (3.54 mi²)

Parameter	Value	Regression Equation Valid Range	
		Min	Max
Drainage Area (square miles)	3.54	0.52	293.7
Mean Basin Slope from 10m DEM ft per ft (feet per foot)	0.0822	0.06632	0.22653

Urban Flows Region Grid Basin Characteristics

100% Undefined Region (3.54 mi²)

The selected watershed is entirely in an area for which flow equations were not defined.

Low Flows Region Grid Basin Characteristics

100% Undefined Region (3.54 mi²)

The selected watershed is entirely in an area for which flow equations were not defined.

Peak Flows Region Grid Statistics

Statistic	Value	Unit	Prediction Error (percent)	Equivalent years of record	90-Percent Prediction Interval	
					Min	Max
PK1 25	89.4	ft ³ /s	24	5.7		
PK1 5	113	ft ³ /s	22	5.9		
PK2	138	ft ³ /s	21	7.1		
PK5	232	ft ³ /s	22	12		
PK10	309	ft ³ /s	24	14		
PK25	423	ft ³ /s	29	15		
PK50	523	ft ³ /s	33	16		
PK100	637	ft ³ /s	37	15		
PK200	768	ft ³ /s	42	15		
PK500	968	ft ³ /s	48	15		

http://www.gishydro.umd.edu/HydroPanel/hydrology_panel_report_3rd_edition_final.pdf

(http://www.gishydro.umd.edu/HydroPanel/hydrology_panel_report_3rd_edition_final.pdf)

Thomas, Jr., W.O. and Moglen, G.E., 2010, An Update of Regional Regression Equations for Maryland, Appendix 3 in Application of Hydrologic Methods in Maryland, Third Edition, September 2010: Maryland State Highway Administration and Maryland Department of the Environment, 38 p.

HY-8 Existing Culvert Analysis Report

Site Data - Culvert 48"

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 999.25 ft

Outlet Station: 34.00 ft

Outlet Elevation: 998.00 ft

Number of Barrels: 1

Culvert Data Summary - Culvert 48"

Barrel Shape: Circular

Barrel Diameter: 4.00 ft

Barrel Material: Corrugated Steel

Embedment: 8.00 in

Barrel Manning's n: 0.0240 (top and sides)

Manning's n: 0.0350 (bottom)

Culvert Type: Straight

Inlet Configuration: Thin Edge Projecting

Inlet Depression: NONE

Tailwater Channel Data - Crossing 1

Tailwater Channel Option: Rectangular Channel

Bottom Width: 18.00 ft

Channel Slope: 0.0200

Channel Manning's n: 0.0350

Channel Invert Elevation: 998.65 ft

Roadway Data for Crossing: Crossing 1

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 30.00 ft

Crest Elevation: 1005.00 ft

Roadway Surface: Paved

Roadway Top Width: 30.00 ft

Table 1 - Culvert Summary Table: Culvert 48"

Discharge Names	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
PK2	73.90	73.90	1002.74	2.828	0.0*	1-S2n	1.928	2.251	1.939	0.824	10.187	4.980
PK10	168.00	113.23	1005.71	4.063	6.902	7-M2c	2.817	2.817	2.817	1.379	11.104	6.766
PK100	356.00	121.10	1006.88	4.371	6.965	7-M2c	3.333	2.898	2.898	2.234	11.637	8.852

Table 2 - Downstream Channel Rating Curve (Crossing: Crossing 1)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
73.90	999.47	0.82	4.98	1.03	0.97
168.00	1000.03	1.38	6.77	1.72	1.02
356.00	1000.88	2.23	8.85	2.79	1.04

Table 3 - Summary of Culvert Flows at Crossing: Crossing 1

Headwater Elevation (ft)	Discharge Names	Total Discharge (cfs)	Culvert 48" Discharge (cfs)	Roadway Discharge (cfs)	Iterations
1002.74	PK2	73.90	73.90	0.00	1
1005.71	PK10	168.00	113.23	54.73	5
1006.88	PK100	356.00	121.10	234.93	5
1005.00	Overtopping	113.23	113.23	0.00	Overtopping

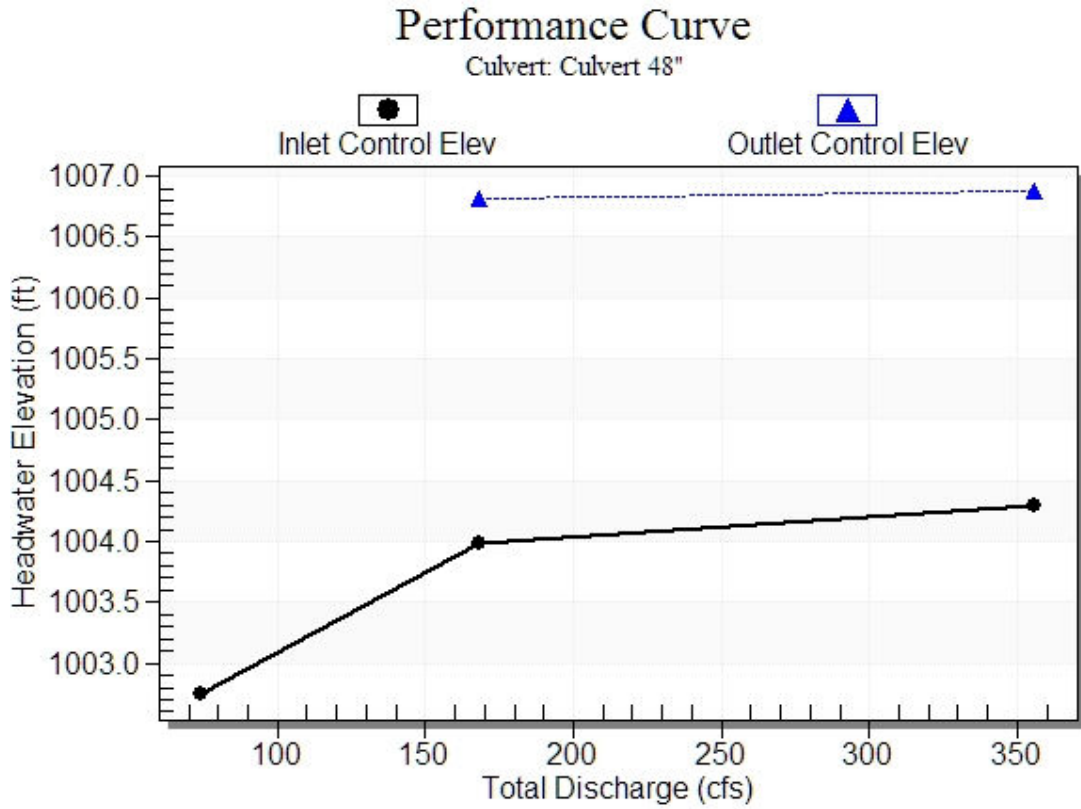


Figure 1 - Culvert Performance Curve Plot: Culvert 48"

Crossing - Crossing 1, Design Discharge - 356.0 cfs

Culvert - Culvert 48", Culvert Discharge - 121.1 cfs

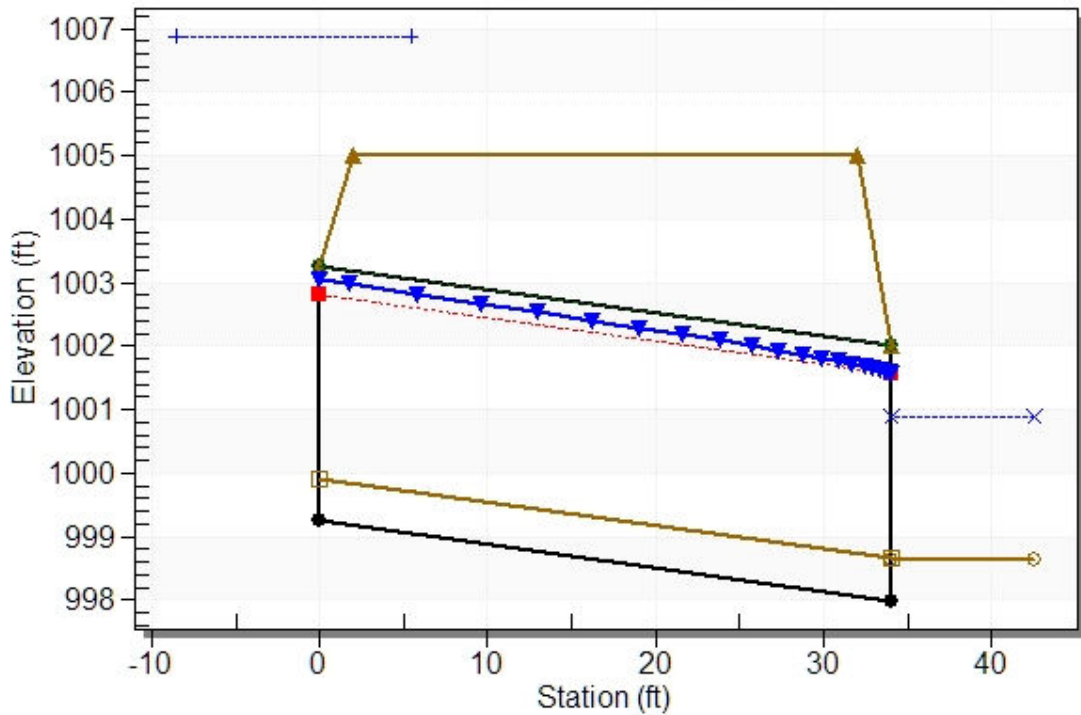


Figure 2 - Water Surface Profile Plot for Culvert: Culvert 48"

Total Rating Curve

Crossing: Crossing 1

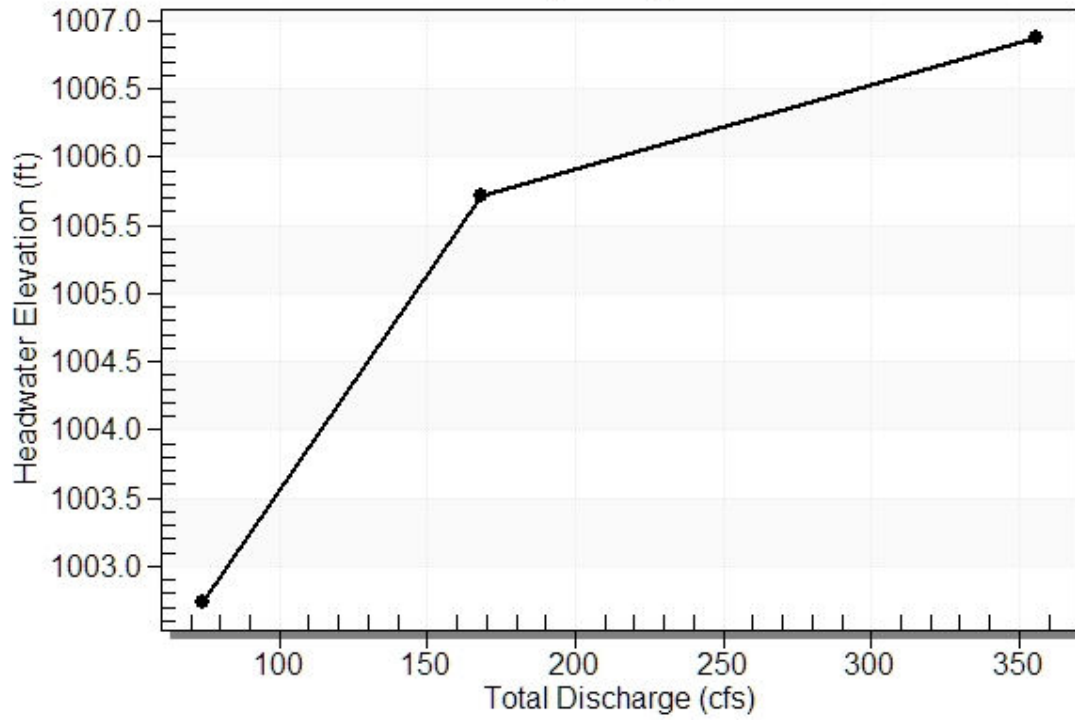


Figure 3 - Rating Curve Plot for Crossing: Crossing 1

HY-8 Proposed Culvert Analysis Report

Site Data - Culvert 36" Double

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 999.25 ft

Outlet Station: 34.00 ft

Outlet Elevation: 998.00 ft

Number of Barrels: 2

Culvert Data Summary - Culvert 36" Double

Barrel Shape: Circular

Barrel Diameter: 3.00 ft

Barrel Material: Concrete

Embedment: 4.00 in

Barrel Manning's n: 0.0120 (top and sides)

Manning's n: 0.0350 (bottom)

Culvert Type: Straight

Inlet Configuration: Square Edge with Headwall

Inlet Depression: NONE

Tailwater Channel Data - Crossing Recommended

Tailwater Channel Option: Rectangular Channel

Bottom Width: 18.00 ft

Channel Slope: 0.0200

Channel Manning's n: 0.0350

Channel Invert Elevation: 998.30 ft

Roadway Data for Crossing: Crossing Recommended

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 30.00 ft

Crest Elevation: 1005.00 ft

Roadway Surface: Paved

Roadway Top Width: 30.00 ft

Table 1 - Culvert Summary Table: Culvert 36" Double

Discharge Names	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
PK2	73.90	73.90	1001.60	2.020	1.551	1-S2n	1.254	1.776	1.311	0.824	10.491	4.980
PK10	168.00	168.00	1003.56	3.982	0.096	5-S2n	2.213	2.528	2.270	1.379	13.903	6.766
PK100	356.00	179.77	1006.55	4.296	0.951	5-S2n	2.450	2.450	2.450	2.234	14.372	8.852

Table 2 - Downstream Channel Rating Curve (Crossing: Crossing Recommended)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
73.90	999.12	0.82	4.98	1.03	0.97
168.00	999.68	1.38	6.77	1.72	1.02
356.00	1000.53	2.23	8.85	2.79	1.04

Table 3 - Summary of Culvert Flows at Crossing: Crossing Recommended

Headwater Elevation (ft)	Discharge Names	Total Discharge (cfs)	Culvert 36" Double Discharge (cfs)	Roadway Discharge (cfs)	Iterations
1001.60	PK2	73.90	73.90	0.00	1
1003.56	PK10	168.00	168.00	0.00	1
1006.55	PK100	356.00	179.77	176.22	5
1005.00	Overtopping	189.33	189.33	0.00	Overtopping

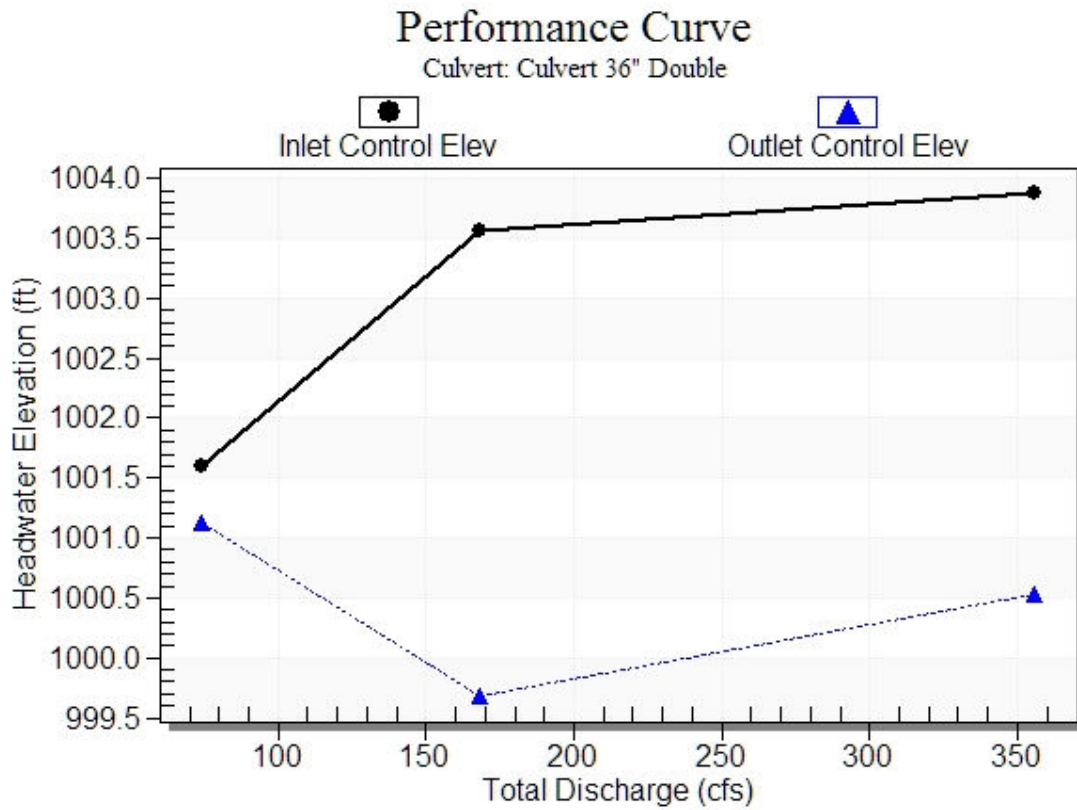


Figure 1 - Culvert Performance Curve Plot: Culvert 36" Double

Crossing - Crossing Recommended, Design Discharge - 356.0 cfs

Culvert - Culvert 36" Double, Culvert Discharge - 179.8 cfs

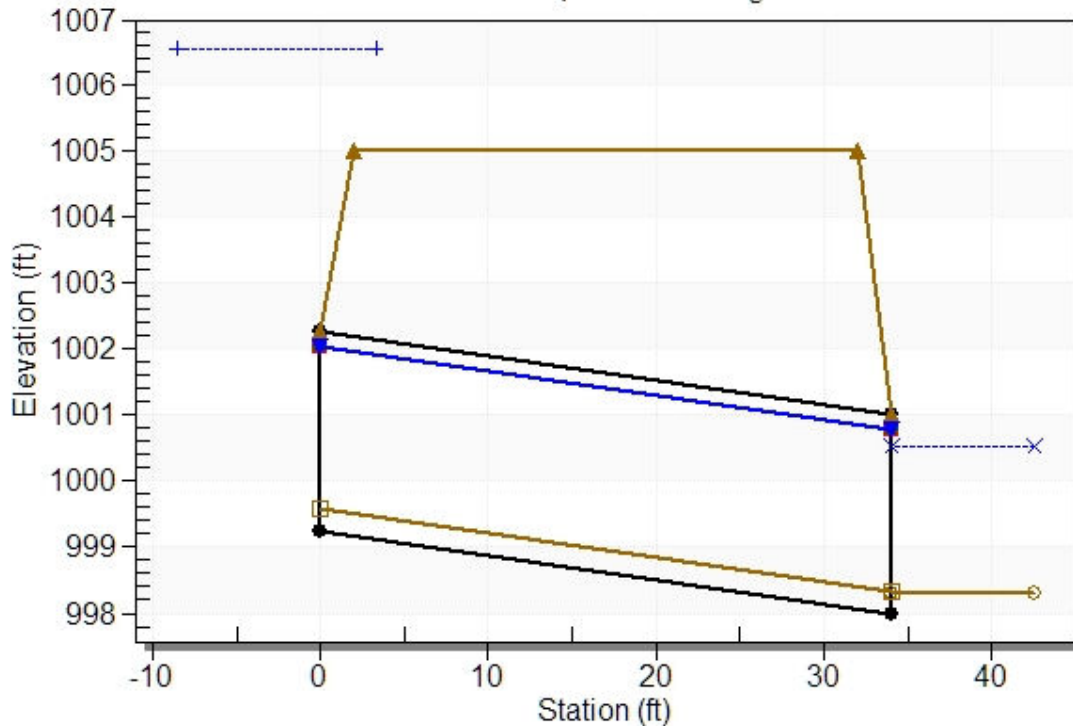


Figure 2 - Water Surface Profile Plot for Culvert: Culvert 36" Double

Total Rating Curve

Crossing: Crossing Recommended

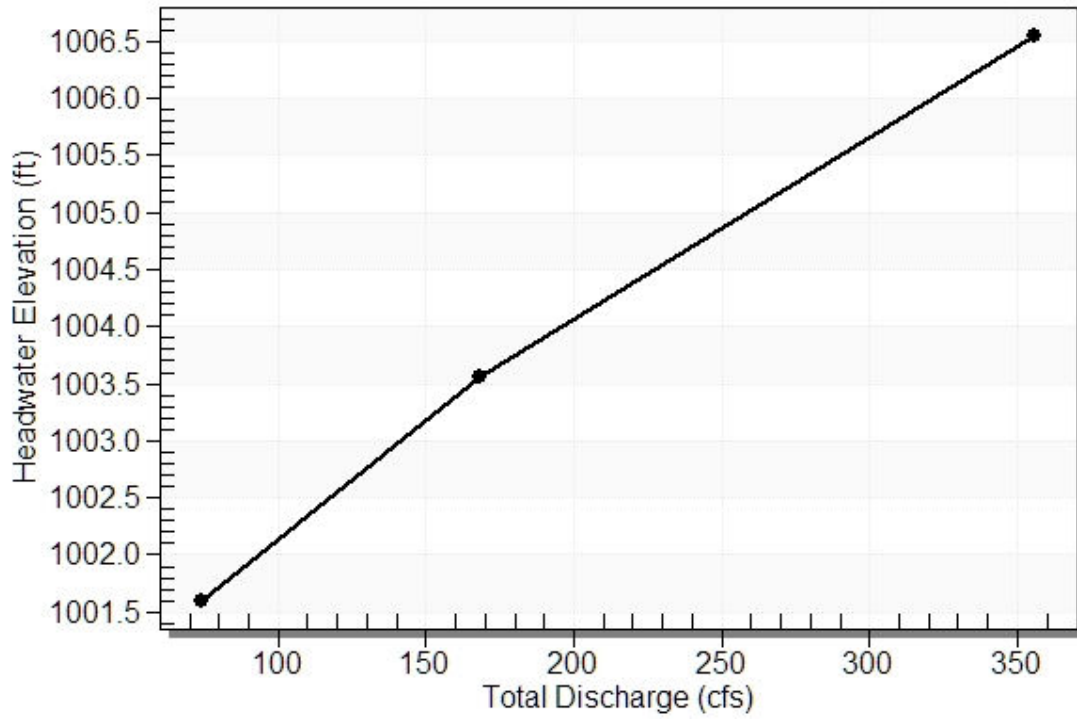


Figure 3 - Rating Curve Plot for Crossing: Crossing Recommended